

STRUCTURAL-GENERAL NOTES

DESIGN CRITERIA AND LOADS

1. WIND DESIGN	
WIND SPEED (MPH)	V(ADJ)=127 V(ULT)=160
RISK CATEGORY	II
EXPOSURE CATEGORY	C
ENCLOSURE CLASSIFICATION	OPEN
INTERNAL PRESSURE COEFFICIENT	CPI = +0.00
TOPOGRAPHIC FACTOR	KZT = 1.0

2. THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE STRUCTURAL ENGINEER OF RECORD (SER) OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL/ STRUCTURAL/ MECHANICAL/ ELECTRICAL OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 100 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

GENERAL REQUIREMENTS

- 1. PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENT INFORMATION IN THE STRUCTURAL GENERAL NOTES.
- 2. THE DESIGN AND CONSTRUCTION OF THIS PROJECT IS GOVERNED BY THE "FLORIDA BUILDING CODE (FBC), SIXTH EDITION, HEREAFTER REFERRED TO AS THE FBC, AS ADOPTED AND MODIFIED BY THE AUTHORITY HAVING JURISDICTION (AHJ).
- 3. WHERE OTHER STANDARDS ARE NOTED IN THE DRAWINGS, USE THE LATEST EDITION OF THE STANDARD UNLESS A SPECIFIC DATE IS INDICATED. REFERENCE TO A SPECIFIC SECTION IN A CODE DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE ENTIRE STANDARD.
- 4. REFER TO THE CIVIL DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING BUT NOT LIMITED TO: DIMENSIONS, ELEVATIONS, SLOPES, DOOR AND WINDOW OPENINGS, NON-BEARING WALLS, STAIRS, FINISHES, DRAINS, WATERPROOFING, RAILINGS, CURTAIN WALLS, DEPRESSIONS, MECHANICAL UNIT LOCATIONS, AND OTHER NONSTRUCTURAL ITEMS.
- 5. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK WITH ARCHITECT, ENGINEER(S) AND OTHER TRADES; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS; FOR SELECTING FABRICATION PROCESSES; FOR TECHNIQUES OF ASSEMBLY; AND FOR PERFORMING WORK IN A SAFE AND SECURE MANNER.
- 6. IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS/PLAN/DETAILS, REFERENCE STANDARDS, THE ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT/ENGINEER AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.
- 7. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH).
- 8. ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE REVIEWED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.
- 9. ALL BUILDING SITES SHALL BE GRADED TO PROVIDE DRAINAGE UNDER ALL PORTIONS OF THE BUILDING AND AROUND THE BUILDING PERIMETER TO ALLOW DRAINAGE AWAY FROM THE STRUCTURE.
- 10. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY CLOUDED AND NOTED. ARCHITECT/ENGINEER REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL.
- 11. DISCREPANCIES, OMISSIONS, OR INCONSISTENCIES WITH APPLICABLE CODE REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE SUBMITTING A BID OR PROCEEDING WITH THE WORK.
- 12. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATIONS, SHORING AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NEARLY COMPLETE.
- 13. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER IN WRITING BEFORE PROCEEDING WITH THE WORK.

STRUCTURAL CERTIFICATION

- 1. I CERTIFY THAT THE PLANS AND SPECIFICATIONS COMPLY WITH THE STRUCTURAL PORTION OF THE FLORIDA BUILDING CODE SIXTH EDITION.
- 2. I ALSO CERTIFY THAT STRUCTURAL ELEMENTS DEPICTED ON THESE PLANS PROVIDE ADEQUATE RESISTANCE TO THE WIND LOADS SPECIFIED IN SECTION 1609 IN THE FBC AND CHAPTER 3 IN THE FBC-RESIDENTIAL.

FOUNDATION AND SLABS ON GRADE

- 1. FOUNDATION IS DESIGNED BASED ON PRESUMPTIVE SAFE ALLOWABLE BEARING PRESSURE OF 1,500 PSF. CONTRACTOR SHALL VERIFY THAT THE MINIMUM BEARING PRESSURE IS OBTAINED PRIOR TO FOOTING PLACEMENT.
- 2. REINFORCED FOUNDATION REQUIREMENTS USED IN THE DESIGN:
 - a. MINIMUM DEPTH BELOW FINISHED GRADE 1'-0"
 - b. MAXIMUM ALLOWABLE BEARING CAPACITY 1,500 PSF
 - c. MODULUS OF SUBGRADE REACTION 200 PCI
 - d. PASSIVE LATERAL PRESSURE 250 PSF
 - e. ACTIVE LATERAL PRESSURE (UNRESTRAINED) 65 PSF
 - f. ACTIVE LATERAL PRESSURE (RESTRAINED) 35 PSF
 - g. COEFFICIENT OF SLIDING FRICTION 0.4
- 3. ALL FOUNDATION CONCRETE SHALL BE CAST IN THE DRY. DEWATERING OPERATION SHALL BE DONE IN SUCH A WAY THAT GROUND WATER LEVELS OUTSIDE THE SITE WILL BE MAINTAINED TO AVOID SETTLEMENT AND DAMAGE TO NEARBY BUILDINGS AND STRUCTURES.
- 4. SYNTHETIC FIBER REINFORCEMENT SHALL COMPLY WITH ASTM-C-1116, AND THE DOSAGE AMOUNT SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION BUT NOT LESS THAN 1.2 LBS/CY.
- 5. WELDED WIRE FABRIC SHALL CONFORM TO A.S.T.M. A185 (LATEST EDITION), AND BE SUPPORTED ON SLAB CHAIRS SPACED AT 3'-0" ON CENTER MAXIMUM.
- 6. THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE THE MINIMUM COMPRESSIVE STRENGTHS AT 28 DAYS AND THE WATER/CEMENT RATIO IS THE MAXIMUM. THE SPECIFIED SLUMP IS THE MAXIMUM PRIOR TO THE ADDITION OF ADMIXTURES. CONCRETE SHALL BE STANDARD WEIGHT CONCRETE (145 PCF).

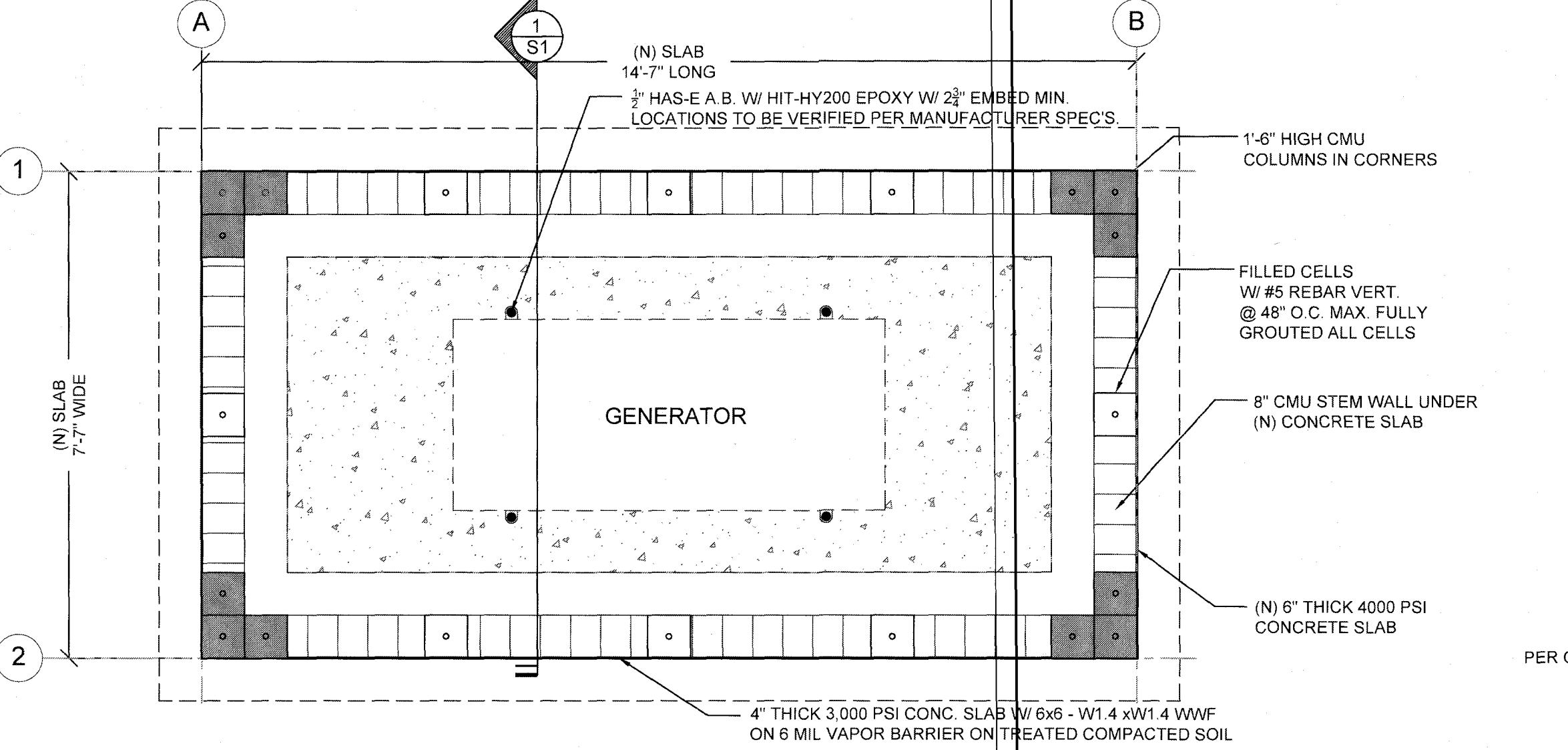
ITEM OF CONSTRUCTION	STRENGTH (PSI)	AGG (IN)	SLUMP (IN)	WATER/CEMENT (L/B/LB)
FOUNDATION PADS	4,000	1	2-4	0.50

- 7. REMOVE AND REPLACE MINIMUM 1 FEET OF EXISTING SOIL BELOW FOUNDATION WITH COMPACTED, MOISTURE-TREATED, NON-EXPANSIVE FILL MATERIAL. FILL AREA SHALL EXTEND 1 FOOT BEYOND FOUNDATION FOOTPRINT.
- 8. FOR SITE PREPARATION, REMOVE DELETERIOUS MATERIAL SUCH AS VEGETATION, ORGANIC SOILS AND ROOT ZONES, EXISTING FILL, OR LOOSE, SOFT FROZEN, OR OTHERWISE UNSUITABLE MATERIALS FROM BELOW THE PROPOSED FOUNDATION AREAS.
- 9. SOIL BENEATH FOOTINGS SHALL BE EXCAVATED AS REQUIRED TO REMOVE ALL ORGANIC AND DELETERIOUS MATERIALS. PLACE CLEAN SAND FILL IN MAXIMUM OF 12 INCH LIFTS. SUBGRADE AND EACH LIFT SHALL BE COMPACTED TO A MINIMUM OF 95 PERCENT OF ITS MODIFIED PROCTOR VALUE IN ACCORDANCE WITH ASTM D 1557.
- 10. SUBGRADE SHALL BE UNIFORM OVER THE ENTIRE FOUNDATION AREA.
- 11. FOUNDATIONS SHALL BEAR ON EITHER COMPETENT NATIVE SOIL OR COMPACTED STRUCTURAL FILL AS PER THE GEOTECHNICAL REPORT. EXISTING PERIMETER FOOTINGS SHALL BEAR NOT LESS THAN 24 INCHES BELOW FINISH GRADE. UNLESS OTHERWISE SPECIFIED BY THE GEOTECHNICAL ENGINEER AND/OR THE BUILDING OFFICIAL.
- 12. PROVIDE 6 MIL 'VISOQUEEN' VAPOR BARRIER UNDER ALL SLABS ON FILL (UNLESS OTHERWISE NOTED ON PLANS).
- 13. NON-EXPANSIVE BACKFILL SHALL BE PLACED IN CONTROLLED LIFTS NOT TO EXCEED 12 INCHES AND SHALL BE COMPACTED TO AT LEAST 95% OF LABORATORY MAXIMUM DENSITY (ASTM D 1557).
- 14. AREA DRAINAGE SHALL BE DIRECTED AWAY FROM THE FOUNDATION.
- 15. GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR SHORING, SHEETING AND BRACING OF EXCAVATIONS.
- 16. IN NO CASE SHALL TRUCKS, BULLDOZERS OR OTHER HEAVY EQUIPMENT BE PERMITTED CLOSER THAN 5 FEET FROM ANY FOUNDATION WALL UNLESS APPROVED BY ENGINEER.

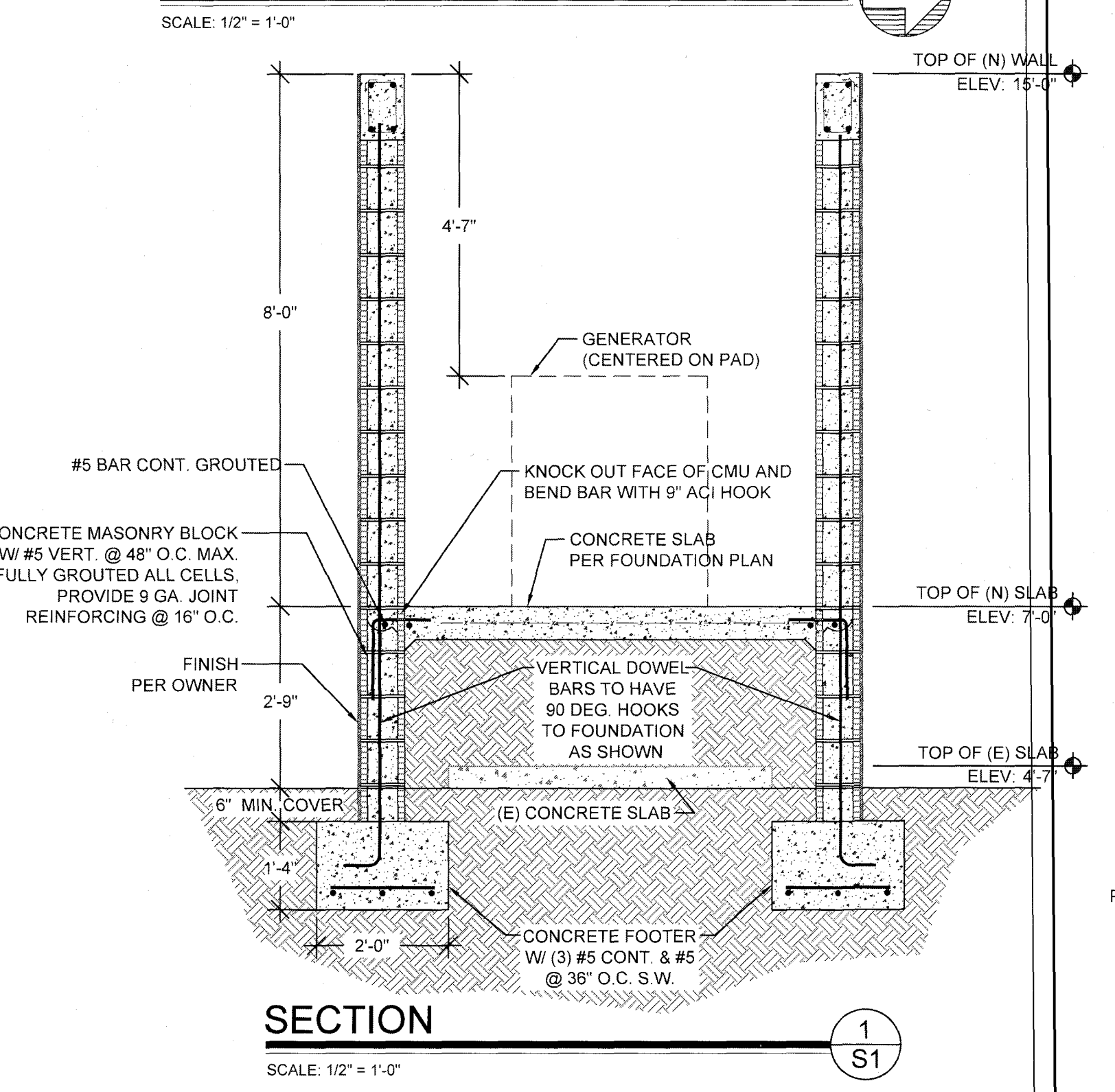
- 1. REQUIREMENTS FOR MASONRY, AND REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES ACI 530/ASCE 6 (LATEST EDITION).
- 2. ALL MASONRY WORK SHALL CONFORM TO ACI 530/ASCE 6 STANDARDS, LATEST EDITION.
- 3. MOISTURE CONTENT OF BLOCKS SHALL NOT EXCEED 35% OF TOTAL ABSORPTION AT THE TIME OF PLACEMENT.
- 4. BLOCK UNITS SHALL CONFORM TO FLORIDA CONCRETE AND PRODUCTS ASSOCIATION SPECIFICATION "CM-1".
- 5. MAXIMUM LINEAR SHRINKAGE FOR BLOCK UNITS USED FOR EXTERIOR WALL SHALL NOT EXCEED 0.4%.
- 6. CONCRETE MASONRY UNITS SHALL BE IN CONFORMANCE WITH ASTM C 90, GRADE N, TYPE II. MASONRY UNITS SHALL BE TESTED IN ACCORDANCE WITH ASTM C 140 AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI MINIMUM BASED ON THE NET CROSS SECTIONAL AREA.
- 7. TESTING TO BE DONE FOLLOWING ASTM C 140 "SAMPLING AND TESTING OF CONCRETE MASONRY UNITS".
- 8. USE ALL GROUT CONFORMING TO ASTM C 476 WITH A MIN. COMPRESSIVE STRENGTH OF 3000 PSI IN 28 DAYS. TESTED IN ACCORDANCE WITH ASTM C 39. COARSE TYPE WITH MAX. AGGREGATE SIZE OF 3/8" AND SLUMP OF 8" TO 11". TEST SAMPLES FOR COMPRESSIVE STRENGTH EVERY 30 YARDS OR EA DAY OF GROUTING. 2,800 PSI PUMP MIX READY MIX CONCRETE MADE WITH MAX. 3/8" AGGREGATE AND MAX. 9" SLUMP IS ACCEPTED ALTERNATE. NO ADMIXTURES WILL BE PERMITTED IN MORTAR.
- 9. GROUT FOR POURING SHALL BE A FLUID CONSISTENCY.
- 10. USE TYPE "M" MORTAR IN CONFORMANCE WITH ASTM C 270, AND ASTM C 780. TYPES (DO NOT USE MASONRY CEMENT). MORTAR SHALL BE FRESHLY PREPARED AND UNIFORMLY MIXED.
- 11. REMOVE MORTAR PROTRUDING INTO CELL CAVITIES THAT ARE TO BE REINFORCED AND GROUTED. ALLOW A MIN. OF 24 HOURS FOR MORTAR TO CURE BEFORE PLACING GROUT.
- 12. REINFORCING STEEL SHALL CONFORM TO ASTM A 615 GRADE 60, F_y = 60,000 PSI. ALL SPLICES (LAPS) AND CORNER BARS SHALL BE MINIMUM 30 INCHES OR AS SHOWN ON DRAWINGS. EPOXY COATED BARS SHALL HAVE THEIR LAP LENGTHS 90% GREATER THAN THOSE SPECIFIED ABOVE.
- 13. ANCHOR BOLTS SHALL BE ASTM A 307, FOR HEADED MACHINE BOLTS.
- 14. DO NOT STACK MASONRY UNITS MORE THAN 2'-8" HIGH AND IN PALLETES OF 4'x4' MAXIMUM SURFACE AREA AND NO LESS THAN 8 FT. AWAY FROM EACH OTHER.
- 15. ALL UNITS TO BE LAID UP IN RUNNING BOND WITH CONCAVE COMPRESSED JOINTS UNLESS NOTED OTHERWISE.
- 16. HEAD AND BED JOINTS SHALL BE 3/8" THICK EXCEPT STARTING JOINT AT FOUNDATION WHICH SHALL BE 1/4" MINIMUM AND 3/4" MAXIMUM. ALL UNITS SHALL BE LAID WITH FULL MORTAR COVERAGE OF THE FACE SHELLS IN BOTH HORIZONTAL AND VERTICAL OR TRUSS.
- 17. PROVIDE 9 GAUGE LADDER TYPE CONT. GALVANIZED HORIZONTAL JOINT REINFORCING (DUR O WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES (16" VERTICALLY) WITH MIN. 8" LAP SPLICE.
- 18. USE PREFABRICATED CORNERS AND TEES AT WALL INTERSECTIONS. OVERLAP DISCONTINUOUS ENDS A MIN. OF 12". HORIZONTAL REINFORCING SHALL CONFORM TO ASTM A-82.
- 19. INTERSECTING WALLS SHALL BE INTERLOCKED WITH RUNNING BOND.
- 20. 18 GAUGE DOVETAIL ANCHORS (5-8" LONG) AND INSERTS SHALL BE USED EVERY 2ND BLOCK COURSE AT BLOCK-COLUMN INTERSECTIONS.
- 21. WHERE VERTICAL REINFORCEMENT IS REQUIRED PROVIDE ONE PIECE, NO SPLICES, CENTERED IN THE WALL UNLESS SPECIFICALLY DETAILED. OTHERWISE PROVIDE VERTICAL SUPPORT SPACERS AT 200 REINFORCEMENT DIAMETERS MAXIMUM BUT NOT EXCEEDING 10 FEET. THE CONTRACTOR HAS THE OPTION TO USE ADDITIONAL LAP SPLICES FOR THE PLACEMENT OF THE VERTICAL REINFORCING.
- 22. ALL REINFORCED HOLLOW UNIT MASONRY SHALL BE BUILT TO PRESERVE THE UNOBSTRUCTED VERTICAL CONTINUITY OF THE CELLS TO BE FILLED. WALLS AND CROSS WEBS FORMING SUCH CELLS TO BE FILLED SHALL BE FULL-BEDDED IN MORTAR TO PREVENT LEAKAGE OF GROUT. ALL HEAD (OR END) JOINTS SHALL BE SOLIDLY FILLED WITH MORTAR FOR A DISTANCE IN FROM THE FACE OF THE WALL OR UNIT NOT LESS THAN THE THICKNESS OF THE LONGITUDINAL FACE SHELLS. BOND SHALL BE PROVIDED BY LAPPING UNITS IN SUCCESSIVE VERTICAL COURSES OR BY EQUIVALENT MECHANICAL ANCHORAGE.
- 23. VERTICAL CELLS TO BE FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL CELL MEASURING NOT LESS THAN 3" AND HAVING A CLEAR AREA OF 10 SQUARE INCHES.
- 24. REINFORCING BARS REQUIRE A MINIMUM CLEAR DISTANCE OF 1/4" FOR FINE GROUT OR 1/2" FOR COURSE GROUT BETWEEN ANY MASONRY FACE. A MINIMUM 2" COVER FROM THE EXTERIOR FACE OF THE BLOCK TO THE REINFORCING INCLUDING GROUT SHALL ALSO BE OBSERVED.
- 25. FOR REINFORCING CONGESTION KNOCK OUT BLOCKS TO BE USED TO FACILITATE CONSTRUCTION.
- 26. USE MINIMUM 1 #5 IN FILLED CELL AT WALL INTERSECTIONS, EACH SIDE OF OPENINGS IN THE WALL AND AT THE CORNERS OF WALLS UNLESS NOTED IN PLANS TO BE DIFFERENT.
- 27. ALL CELLS CONTAINING REINFORCING OR EMBEDDED ITEMS SHALL BE SOLID GROUTED. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF ALL CELLS TO BE FILLED IN EACH COURSE. POUR OF GROUT WHERE SUCH GROUT POUR IS IN EXCESS OF 4 FEET IN HEIGHT. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS. THE CLEANOUT SHALL BE SEALED BEFORE GROUTING, AFTER INSPECTION.
- 28. GROUT SHALL BE A CONTINUOUS OPERATION Poured IN LIFTS OF 8 FEET MAXIMUM HEIGHT. ALL GROUT SHALL BE CONSOLIDATED AT TIME OF POURING BY PUDDLING OR VIBRATION AND THEN RECONSOLIDATED AGAIN BY PUDDLING LATER, BEFORE PLASTICITY IS LOST.
- 29. WHEN TOTAL GROUT POUR EXCEEDS 8 FEET IN HEIGHT, THE GROUT SHALL BE PLACED IN FOUR FOOT LIFTS WITH NOT LESS THAN 30 MINUTES NOR MORE THAN ONE HOUR BETWEEN LIFTS. VIBRATE EACH LIFT AND RECONSOLIDATE PREVIOUS LIFT AFTER PLACING NEXT.
- 30. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE POUR OF GROUT NOT LESS THAN 1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.
- 31. UNITS WHICH ARE DISTURBED AFTER INITIAL BOND IS ACHIEVED MUST BE REMOVED AND RELAI D WITH FRESH MORTAR TO ENSURE ADEQUATE BOND STRENGTH AND MINIMIZE THE LIKELIHOOD OF WATER PENETRATION INTO AN UNBONDED JOINT.
- 32. WHERE ANCHOR BOLTS ARE SET IN MASONRY WALL, FILL BLOCK CELLS WITH GROUT FOR BOLT COURSE, ONE COURSE ABOVE AND TWO COURSES BELOW ANCHOR ELEVATION.
- 33. CHASES AND RECESSES SHALL BE CONSTRUCTED AS MASONRY UNITS ARE LAID. MASONRY DIRECT ABOVE CHASES OR RECESSES WIDER THAN 12 INCHES SHALL BE SUPPORTED ON PRECAST GROUTED LINTELS.
- 34. FOR SPECIAL INSPECTIONS THE ENGINEER SHALL BE GIVEN A MINIMUM 72 HOURS NOTICE PRIOR TO EACH REINFORCED BLOCK GROUTING OR CONCRETING OPERATION.
- 35. ALL COLUMNS NOT SCHEDULED ARE TIE COLUMNS AND SHALL BE 8"x12" MIN. WITH 4 #5 VERTICAL BARS AND #3 TIES AT 12" O.C.

- 1. IS CLEANED UP FROM AROUND THE ANCHOR ROD, SO THAT IT DOES NOT INTERFERE WITH ADJUSTABILITY OF ANCHOR ROD IN SLOTTED HOLE.
- 10. ADHESIVE ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC201 OR AC108.
- 11. ADHESIVE ANCHORS IN MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC201 OR AC108.
- 12. EXISTING REINFORCING BARS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE CUT UNLESS APPROVED BY THE EOR.
- 13. ADHESIVE ANCHORS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE INSTALLED UNTIL THE CONCRETE AND/OR MASONRY HAS BEEN CURED FOR AT LEAST 21-DAYS.
- 14. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL POST INSTALLED ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND THE CURRENT ICC-ES REPORT (IBC2021 TABLE 1705.3 NOTE B).
- 15. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL (ACI 318-11 D.9.2.4).

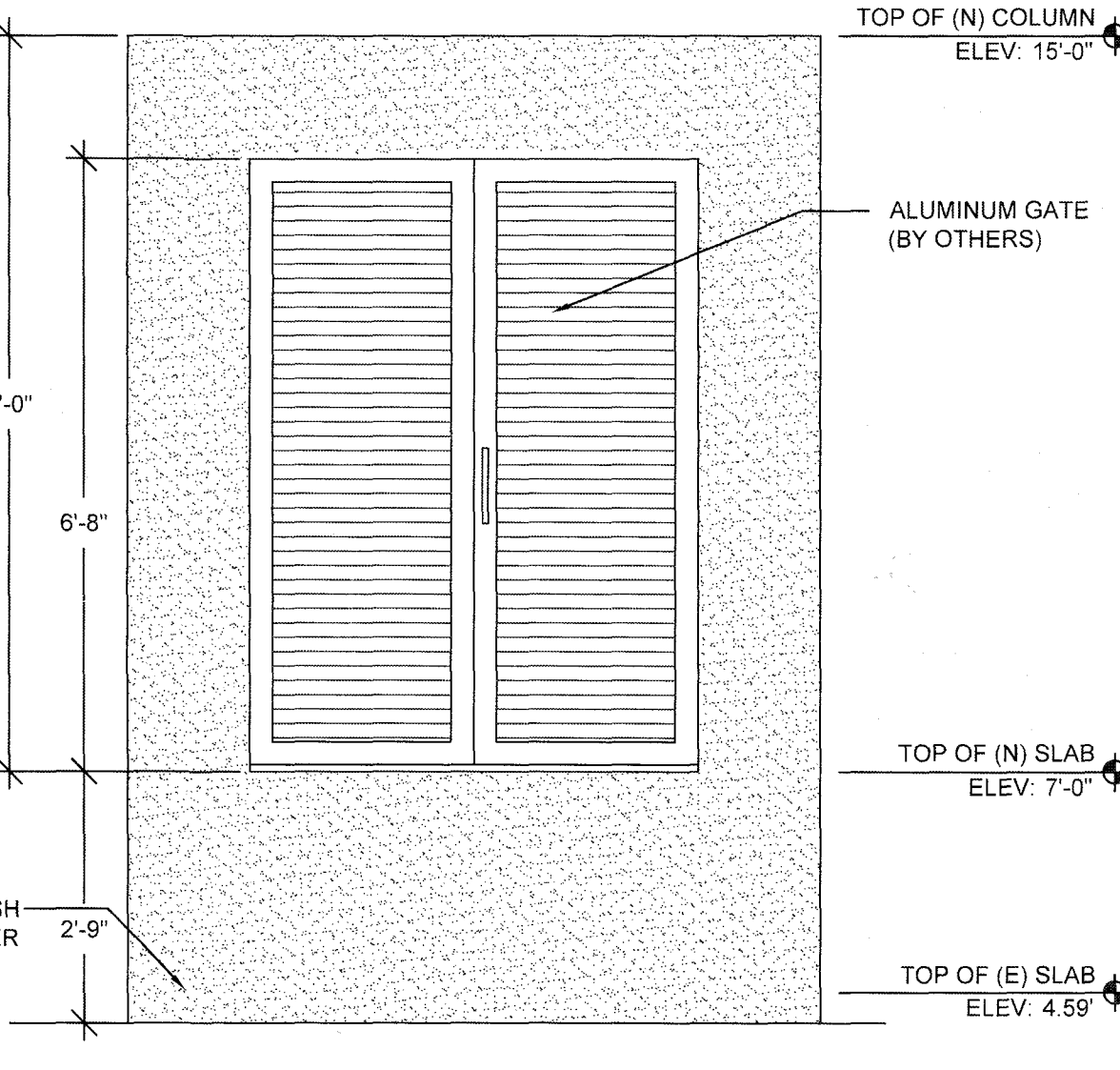
ABBREVIATIONS			
AB	- ANCHOR BOLT	HORIZ	- HORIZONTAL
AFF	- ABOVE FINISHED FLOOR	IN	- INCHES
AHJ	- AUTHORITY HAVING JURISDICTION	MAS	- MASONRY
ALT	- ALTERNATE	MAX	- MAXIMUM
BOTT	- BOTTOM	MFR	- MANUFACTURER
CFS	- COLD FORMED STEEL	MIN	- MINIMUM
CMU	- CONCRETE MASONRY UNIT	MISC	- MISCELLANEOUS
COL	- COLUMN	MHP	- MILES PER HOUR
CONC	- CONCRETE	NTS	- NOT TO SCALE
CONT	- CONTINUOUS	OC	- ON CENTER
D&E	- DRILLED AND EPOXY	PSF	- POUNDS PER SQUARE FOOT
DIA	- DIAMETER	PT	- PRESSURE TREATED
DIM	- DIMENSION	REV	- REVISION/REVISED
DN	- DOWN	SPECS	- SPECIFICATIONS
EA	- EACH	SCHED	- SCHEDULE
ELEV	- ELEVATION/ELEVATOR	TYP	- TYPICAL
ENGR	- ENGINEER	UNO	- UNLESS NOTED OTHERWISE
EW	- EACH WAY	VERT	- VERTICAL
E	- EXISTING	VF	- VERIFY IN FIELD
EXP	- EXPANSION	W	- WITH
EXT	- EXTERIOR	WO	- WITHOUT
FBO	- FLORIDA BUILDING CODE	W/F	- WELDED WIRE FABRIC
FF	- FINISH FLOOR	W/M	- WELDED WIRE MESH
FND	- FOUNDATION	#5	- STEEL REINFORCING BAR (REBAR #5 (5/8")
FT	- FEET/FOOT		
FTG	- FOOTING		



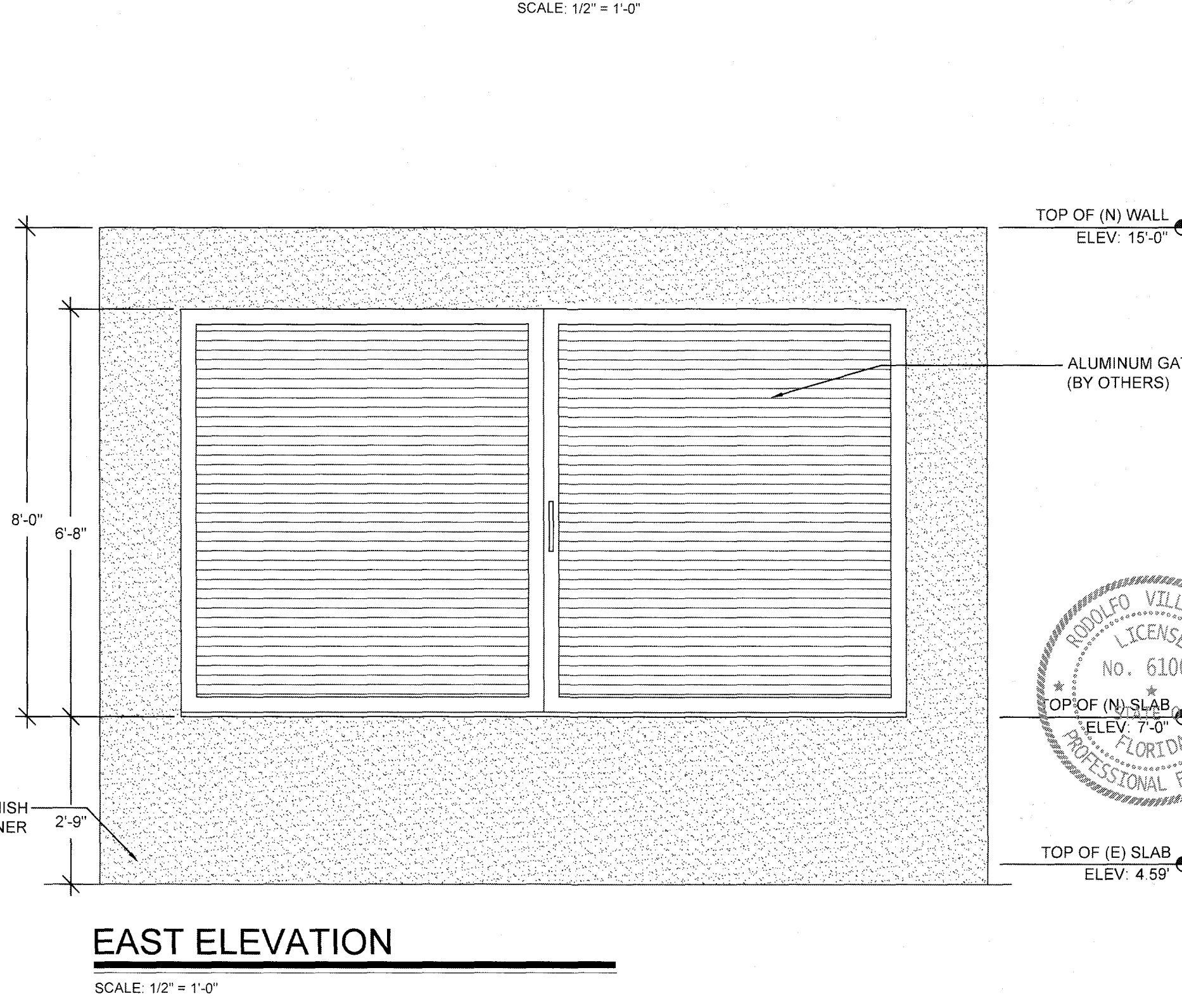
FOUNDATION PLAN
SCALE: 1/2" = 1'-0"



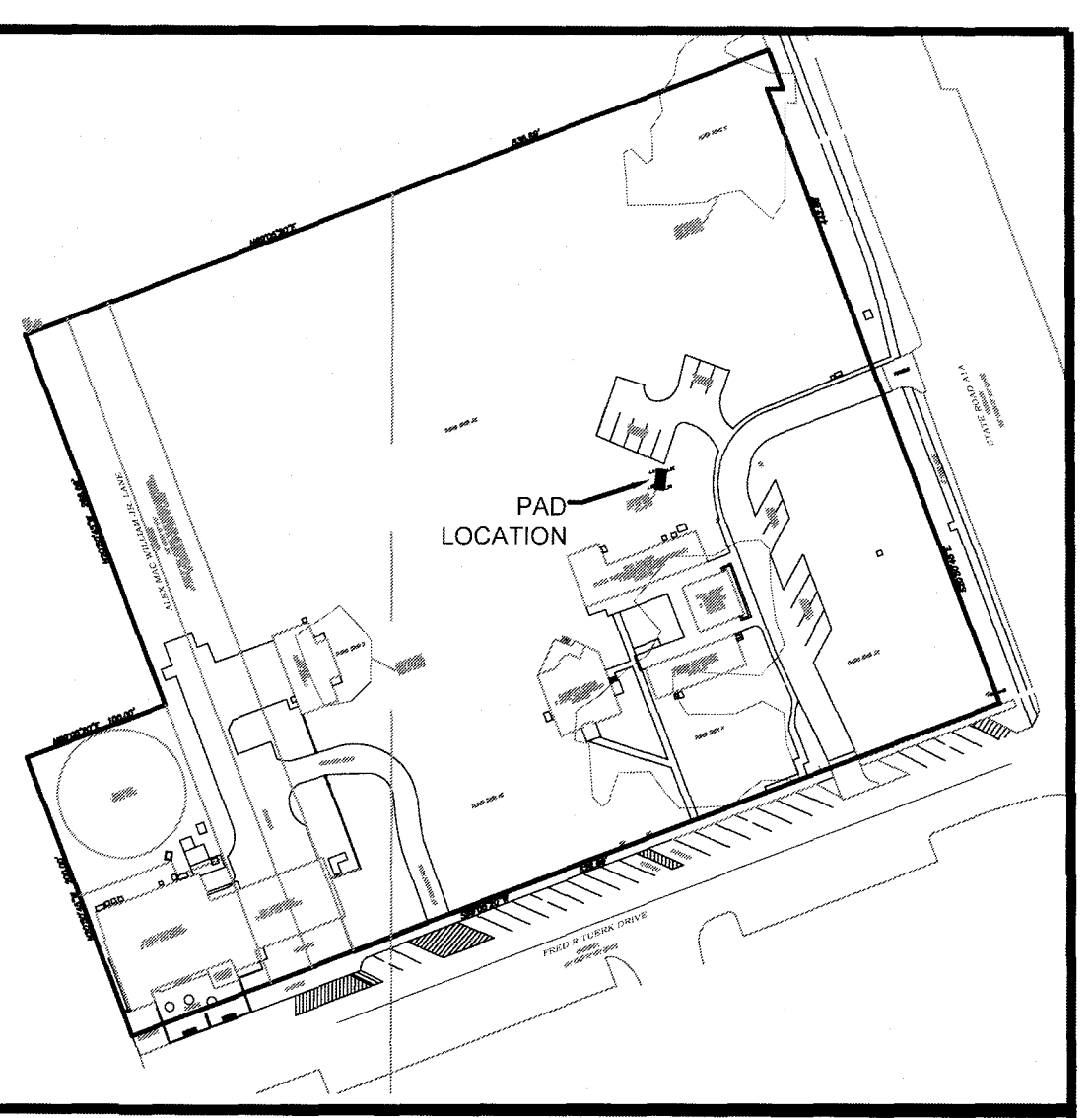
SECTION
SCALE: 1/2" = 1'-0"



NORTH ELEVATION
SCALE: 1/2" = 1'-0"



EAST ELEVATION
SCALE: 1/2" = 1'-0"



LOCATION MAP

REVISIONS

NO.	DATE	BY	DESCRIPTION
1			
2			
3			
4			
5			
6			
7			

JOB NO. 18-0026

DESIGNED: RK
DRAWN: AER
DATE: FEB 2018
CHECKED: RV
DATE ISSUED: 03-13-2018
SCALE: AS NOTED

1835 - 20TH STREET
VERO BEACH, FL 32960
PH. (772) 569-0035
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MBV ENGINEERING, INC.
MORE BOVILLES MARK & ASSOCIATES
CONSULTING ENGINEERING - CA #9726

LOCATION MAP,
FOUNDATION PLAN,
SECTION, AND
STRUCTURAL NOTES

TOWN OF INDIAN RIVER
SHORES TOWN HALL
Generator Pad

FLORIDA

ROLANDO VILLAMIZAR
LICENSE
NO. 61000
TOPOF (N) SLAB
ELEV. 7'-0"
PROFESSIONAL ENGINEER

DATE: 03-13-2018

SHEET
S1
1 OF 1
18-0026

PERMIT SET